APPENDIX H

Stage 1 Site Contamination & Geotechnical Investigation



Mt Kuring-Gai 12/9-15 Gundah Rd Mt Kuring-Gai NSW 2080 T: (02) 8438 0300 F: (02) 8438 0310

Wollongong 1/140 Industrial Rd Oak Flats NSW 2529 T: (02) 4257 4458 F: (02) 4257 4463

E: admin@netgeo.com.au W: www.netgeo.com.au ABN: 35 069 211 561

D.Y. SURVEYORS PROPOSED RESIDENTIAL SUBDIVISION LOT 100 & 101 DP 1157883 **26 DOWNES PLACE, JAMBEROO** PRELIMINARY GEOTECHNICAL ASSESMENT AND **STAGE 1 CONTAMINATION ASSESSMENT**

REPORT W07/1966-Ar JANUARY 2013



Mt Kuring-Gai 12/9-15 Gundah Rd Mt Kuring-Gai NSW 2080 T: (02) 8438 0300 F: (02) 8438 0310 Wollongong 1/140 Industrial Rd Oak Flats NSW 2529 T: (02) 4257 4458 F: (02) 4257 4463

E: admin@netgeo.com.au W: www.netgeo.com.au ABN: 35 069 211 561

W07/1966-Ar MA:ma 17 January, 2013

D.Y. Surveyors PO Box 178 KIAMA NSW 2533

Attention: David Yates

Dear Sir

Re: Proposed Residential Subdivision, Lot 100 & 101 DP 1157883 Downes Place, Jamberoo: Preliminary Geotechnical assessment and Stage 1 Contamination Assessment.

This report presents the findings of a Stage 1 Site Contamination Assessment, and preliminary geotechnical assessment comprising interim AS2870 Lot classification, slope instability assessment and comments on footing design on proposed buildings relevant for the above project.

This report should be read in conjunction with the attached General Notes. Please contact the undersigned should you have any queries.

For and on behalf of Network Geotechnics Pty Ltd

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V W de Silva *BScEng, MEng, SMIE Aust, CPEng, NPER* Principal Geotechnical Engineer

EXECUTIVE SUMMARY

Network Geotechnics Pty Ltd (NG) was commissioned by D.Y. Surveyors to undertake a Geotechnical Assessment for a proposed residential subdivisional at Lots 100 & 101 Downes Place, Jamberoo. The investigation included a Stage 1 Contamination Assessment of the proposed site and geotechnical aspects relevant for development consent for the proposed subdivision. The investigation was carried out in accordance with NG Proposal dated 3rd August, 2012.

The Stage 1 Contamination Assessment for the site was based on walk over inspection, historical title search and review of historical aerial photographs.

Based on the site history, aerial photographs, fieldwork and Laboratory results, the following comments were noted:

- The site has been used mostly for a dairy farm and other farming purposes up to present.
- The aerial photographs and other records reviewed indicated a low risk of potential contamination within the proposed site.

It is assessed that the risk of site contamination to be low and that a Stage 2 detailed contamination assessment would not be required.

The proposed building sites are provided AS2870-2011 lot classification of H1 (Highly reactive) on account of proposed cut/fill earthworks.

It is assessed that the site has a very low risk of landslide risk and land subsidence.

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Test Pit Logs (TP1- TP4), Terms & Symbols (5 sheets) NG Laboratory Results (3 sheets) DRAWING No. W07-1966-1 Site Plan

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1.0 INTRODUCTION

1.1 General

Network Geotechnics Pty Ltd (NG) was commissioned by D.Y. Surveyors to undertake a Geotechnical Assessment for a proposed subdivision at Lots 100 & 101 DP 1157883 Downes Place, Jamberoo. The investigation also included a Stage 1 Contamination Assessment of the site and other geotechnical aspects relevant for the project development consent. The investigation was carried out in accordance with NG Proposal dated 3rd August, 2012.

The geotechnical advice provided by this report is as follows:

- Investigation of subsurface soil and rock profiles across the site, including the presence of groundwater.
- Geotechnical recommendations and lot classification as per AS2870-2011 including footing types.
- Recommendations on drainage for proposed works.
- The potential for site contamination.

A site plan is shown on the attached Drawing No W07/1966-1.

1.2 Scope of Work

The following scope of work was agreed to with the client:

- Desk study involving review of historical land titles and aerial photographs to facilitate identification of potential site contamination.
- Excavating 4 test pits (TP1 to TP4) to 2.0m depth or to prior refusal on rock using a 5 tonne excavator with 600mm tooth bucket attached, logging and sampling for laboratory testing.
- Evaluation of findings and preparation of a Stage 1 Contamination Assessment.
- Geotechnical assessment covering footing design and AS2870-2011 lot classification.

2.0 SITE IDENTIFICATION

The site is located on the southern corner of Downes Place and Drualla Road in Jamberoo. The site is described as Lots 100 & Lot 101 in DP 1157883 and has an approximate area of $6640m^2$. The site is located in Kiama Local Government Area (LGA) and is bounded by:

- Drualla Road to the North
- Downes Place to the east
- Farm land to the West and South.

Based on Satellite photographs, the approximate site co-ordinates are given below:

Easting
Northing

Reference to Cadastral Records Enquiry Report indicates the site is within the Parish of Kiama, County of Camden.

3.0 SITE HISTORY

<u>3.1 Zonina</u>

Council zoning plans identifies the land Lots 100 & 101 in DP 1157883 Downes Place, Jamberoo with **Rural Landscape Zoning (RU2)** based on Kiama LEP 2011. The objective of the zone is to encourage sustainable primary industry production by maintaining and enhancing the natural resource base, to maintain the rural landscape character of the land.

3.2 Land Use

Based on Aerial Photographs and title records, the site has been mostly used as farmland since 1903 to present. William Geoffrey Downes and Margaret Elizabeth Downes have been joint tenants from 1999 to present. Since 1999 the land seems to have been used for residential purposes and some minor farming.

3.3 Council Rezoning

No records were available on rezoning of the site. The site is proposed to be rezoned for Low Density Residential.

3.4 Historical Site Usage

Based on title records and aerial photographs, the following historical uses could be identified:

<u> Table 3.4.1 – Historical Land Use</u>

1903 - Present	Farming/Dairy Farm
1897-1903	Unknown

3.5 Title Records

A record of ownership/leasing as can be inferred from title records is given in Table 3.5.1 below.

<u>Table 3.5.1</u>	Historical Ownership / Leasing	

Date	Lot /DP Number	Volume/Folio	Remarks
17 September, 1897		No. 737 Book. 606	John Conway, occupation farmer, vendor to Australian Bank Limited, Purchaser.
20 May, 1903		No. 343 Book. 735	John Wilson (storekeeper) vendor to Stanley Dudgeon (Farmer)
20 June, 1932		No. 58 Book. 1643	Joseph Smith (farmer) vendor to Arthur William Downes (Farmer) purchaser.
23 November, 1971		Vol. 466 Fol. 3135	Arthur William Downes (Dairy Farmer) to A.W. Downes Pty Ltd (Dairy Farm Company).
22 April, 1999	2/781683, 124 &125/834573		Transferred from A.W. Downes Pty Ltd (in liquidation) to William Geoffrey Downes and Margaret Elizabeth Downes as join tenants

3.6 Aerial Photographs

Aerial photographs for Lots 100 & 101 in DP 1157883 Downes Place, Jamberoo were purchased from NSW Department of Lands. A summary of findings are presented in Table 3.7.1

Year of Photo	Scale	Colour	Site Description
1949	1:30000	Black & White	 Only visible structure near the site is the dairy shed outside the southern corner of site.
1963	1:38000	Black & White	 Three new sheds at the southern corner of Drualla Road and north east to the site.
1974	1:40000	Black & White	 Geoffrey Downes Residence newly built. Three sheds are still visible
1984	1:40000	Black & White	 Geoffrey Downes residence is the only residence within the close vicinity asides for the dairy shed at the southern corner. Three sheds were removed
1993	1:25000	Colour	 Downes Place did not exist New Residences built on the southern corner of Drualla road
2002	1:25000	Colour	 More New Residences built on the southern corner of Drualla road Downes Place was constructed due to new residences New swimming pool constructed on Geoffrey Downes residence
2010	Ref: Google Earth	Colour	No other obvious change

Table 3.6.1 Summary of Aerial Photograph Review

3.7 Potential Contamination

Based on the review of aerial photographs, title records and a walk over assessment at the time of the fieldwork, the site appears to have a low possibility of potential contamination. The only structure present within the site is the residence. Dairy sheds were located outside the proposed subdivision. However the use of fill materials to level the building/shed pads and the gravel access road to the dairy shed is assessed to be sources of potential contamination with unknown contaminants.

There is assessed to be low risks of any contamination migrating in to the site from off site sources.

4.0 SITE CONDITION & SURROUNDING ENVIRONMENT

4.1 Topography

The site is located at the southern corner Downes Place, Jamberoo with a gentle western north western facing slope, as observed from the centre of the site. The site is mostly covered with grass with a several medium size trees and tall grassland and weeds throughout. A small dirt road from Downes Place had been constructed for access to the dairy shed south of the proposed works, which comprises 14mm Gravel (100mm layer) overlying residual CLAY.

4.2 Visible Signs of Contamination

During the site visit on 28^{th} November, 2012 the following observations were made:

- The site contains a single storey brick dwelling and outbuilding with a rain water tank and a work shed on the western corner of the existing dwelling.
- Gravel driveway from Downes Place to the dairy shed which is located south of the proposed works. Access way comprises gravel of unknown origin.
- Majority of the site is covered in grass and appears to have had little disturbance from the remainder of the site.
- Medium to large trees were located on the west and north west of the existing dwelling.
- It is likely that all run-off would flow to the west and north west of the existing dwelling.

There was no visible staining of soils or gravel surfaces. Additionally there were no visible signs of contamination such as bare soil patches and plant stress.

The surface soils mainly contained silty clayey SAND topsoil.

4.3 Flood Potential

It is not likely the land would be subject to flooding. The nearest surface water body is Colyers Creek located about 350m to the south east of the site.

4.4 Geology & Hydrogeology

Reference to Sydney Basin 1:500000 Geological Series Sheet (Special) indicates the site to be underlain by Shoalhaven Group, Gerringong Volcanics which comprise Latite, Tuff, Tuffaceous Sandstone and Sandstone.

The subsurface conditions interpreted from the boreholes (TP1 and TP4) are summarised in Table 4.1:

Table 4.1 Subsurface Lithology of Boreholes Encountered

Layer/Description	<i>Depth to Base of Layer (m)</i>
TOPSOIL: Silty Clayey SAND, fine to medium grained, dark grey	
	0.2
RESIDUAL: CLAY, medium plasticity, light brown.	>0.9 - 1.4
WEATHERED ROCK: SANDSTONE, extremely weathered, orange	20.9 - 1.4
	1.0 - >1.5

Groundwater was not encountered in boreholes TP1 to TP4 during the time of the investigation. It is pointed out that groundwater levels and seepages may fluctuate with variations in rainfall, temperature and other factors.

5.0 LABORATORY TESTING

Laboratory testing comprised two Atterberg Limit and Linear Shrinkage tests and four Field Moisture Tests (FMC) tests on samples recovered from boreholes TP1 to TP4, in order to assess the plasticity characteristics and soil reactivity.

Laboratory test reports are attached at the end of this report and are discussed in Section 7.0.

6.0 CONTAMINATION ASSESSMENT

6.1 Summary of Desk Study

Based on the desktop study, the following observations are made:

- The aerial photographs and other records indicated a low potential for site contamination.
- The site may contain fill in the area occupied by the existing residence and • in work shed located west of the existing residence that could have potential for soil contamination. The access driveway for the dairy shed south of the proposed residences contains gravel from unknown sources that could have potential for soil contamination.

In general, there is assessed to be a low risk of potential contamination over the area outside the existing residence and work shed.

6.2 Test Pit Logs & Visual Assessment

Samples recovered from test pits were examined in NG laboratory. There was no visible discoloration nor any odours that would raise concern for contamination in the samples recovered.

7.0 GEOTECHNICAL PARAMETERS & RECOMMENDATIONS

7.1 General

The generalised subsurface profile within the area investigated may be represented by up to 0.2m topsoil underlain by residual CLAY to depths up to 1.4m overlying extremely weathered Sandstone.

Laboratory field moisture content test results on 4 samples ranged from 12.3% to 21.1%. Liquid limit of two samples tested recorded 68% and 85%. Based on the above, it is assessed the residual clay to be highly reactive. The laboratory results are included in Appendix B and are summarised as follows:

Borehole No/ Depth (m)	LL	PL	PI	LS	FMC (%)
				. –	
TP1 (0.3-0.6m)	68	26	42	17	19.4
TP2 (0.4-0.6m)	-	-	-	-	21.1
TP2 (1.2-1.3m)	-	-	-	-	12.3
TP4 (0.5-0.7m)	85	25	60	20.5	16.2
Note: LL (Liquid Limit),	PL (Plastic Limit),	PI (Plastic	: Index), Fi	MC % (Field Moisi	ture Content), LS

Table 7.1.1 – Laboratory Test Results

S (Linear Shrinkage)

Dynamic Penetration Tests (DCP) on CLAY below 0.2m depth in test pits TP1 and TP3 recorded 9 to 25 blows per 150mm penetration, indicating stiff to very stiff consistency.

7.2 AS2870 Classifications

Surface movement due to reactivity has been assessed assuming a soil suction change at the surface of 1.2pF and a suction change depth of 1.5m.

Based on the above, surface movement due to moisture changes was assessed in accordance with AS2870-2011 "Residential Slabs and Footings." It is assessed that the surface movement to be in the range of 40mm to 60mm for the undeveloped site.

The natural site prior to earthworks is assessed as Class H1 in accordance with AS2870-2011 Residential Slabs and Footings.

The classifications and recommendations presented in this report are provided on the basis that the performance expectations set out in Appendix B of AS2870-2011 are acceptable and that future site maintenance complies with CSIRO Sheet BTF-18, a copy of which is attached. In particular, the site should be maintained in stable moisture conditions by providing adequate drainage.

7.3 Footings

Stiffened raft and other high level footing systems may be considered for the proposed lots and should be designed and constructed in accordance with the requirements of the above Standard.

Placement of reactive fill may increase the severity of classifications. Therefore, advice should be sought if fill earthworks exceeding about 0.4m depth is to be carried out on any of the lots in order to verify that the classification remains valid.

The footing systems should be designed with generous provision for structural articulation to reduce potential effects of differential movement between areas of varying fill thickness and residual soil and rock exposed by earthworks.

7.4 Landslide Risk and Subsidence

The risk of landslide is assessed to be very low based on the overall ground slopes and depth to bed rock of about 1m.

The site is not known to have been subjected to underground mining and therefore the risk of subsidence is assessed to be very low.

There was no evidence the site contains uncontrolled fill in excess of 600mm depth or cuts deeper than 600mm.

Any proposed cuttings not more than 1.5m height should be formed not steeper than 2H:1V for short term stability.

Cuts/fill deeper than 1.5m should be supported by engineer designed retaining walls assuming a coefficient of earth pressure at rest of 0.55 and an angle of friction for Clay of 25degrees.

Any permanent unsupported batters less than 1.5m height should be formed not steeper than 3H:1V and protected by suitable ground cover or geofabric.

<u>7.5 Drainage</u>

Drainage measures that should be implemented at this site include:

- Provision of concrete lined cut-off drains (or similarly lined drains) at the crest of batters over 1.5 m in height.
- Provision of subsurface drainage behind any retaining walls.
- All stormwater should be collected and discharged from the site via pipes into designated drainage paths and not allowed to flow on to the ground.
- Subsoil drains under the kerbs on proposed roads.

The site should be maintained in good condition without allowing water ponding. Any landscape features to be constructed such as ponds, retaining walls, etc, should be approved by a geotechnical consultant.

8.0 CONCLUSION

Based on the desk study, field investigation and laboratory tests the following conclusions are made:

- The site is assessed to be of low risk of contamination. It is assessed that a Stage 2 Contamination Assessment is not required for the proposed subdivision.
- Stripping of topsoil for construction of the subdivision should be carried out under the supervision of an experienced consultant and any foreign objects/materials identified should be subjected to further investigation.
- The proposed building sites are provided interim AS2870-2011 classification of Class H1 (Highly reactive) classification.

This report should be read in conjunction with the attached General Notes. Please contact the undersigned if you require further assistance.

For and on behalf of Network Geotechnics Pty Ltd

Reviewed by

Mehran Asadabadi *B.Eng (Civil)* Geotechnical Engineer

V W de Silva *BScEng*, *MEng*, *SMIE Aust*, *CPEng NPER* Principal Geotechnical Engineer



GENERAL NOTES

GENERAL

Geotechnical reports present the results of investigations carried out for a specific project and usually for a specific phase of the project (e.g. preliminary design). The report may not be relevant for other phases of the project (e.g. construction), or where project details change.

SOIL AND ROCK DESCRIPTIONS

Soil and rock descriptions are based on AS 1726 – 1993, using visual and tactile assessment except at discrete locations where field and / or laboratory tests have been carried out. Refer to the terms and symbols sheet for definitions.

GROUNDWATER

The water levels indicated on the logs are taken at the time of measurement and depending on material permeability may not reflect the actual groundwater level at those specific locations. Also, groundwater levels can vary with time due to seasonal or tidal fluctuations and construction activities.

INTERPRETATION OF RESULTS

The discussion and recommendations in the accompanying report are based on extrapolation / interpolation from data obtained at discrete locations. The actual interface between the materials may be far more gradual or abrupt than indicated. Also, actual conditions in areas not sampled may differ from those predicted.

CHANGE IN CONDITIONS

Subsurface conditions can change with time and can vary between test locations. Construction operations at or adjacent to the site and natural events such as floods, earthquakes or groundwater fluctuations can also affect subsurface conditions.

REPRODUCTION OF REPORTS

This report is the subject of copyright and shall not be reproduced either totally or in part without the express permission of this firm. Where information from the accompanying report is to be included in contract documents or engineering specification for the project, the entire report should be included in order to minimise the likelihood of misinterpretation from logs.

FURTHER ADVICE

Network Geotechnics would be pleased to further discuss how any of the above issues could affect your specific project. We would also be pleased to provide further advice or assistance including:

- assessment of suitability of designs and construction techniques;
- contract documentation and specification;
- construction control testing (earthworks, pavement materials, concrete);
- construction advice (foundation assessments, excavation support).

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TERMS AND SYMBOLS

SOIL DES						
	CRIPTIONS			FZ Fracture SZ Shear zo		Stepped Irregular
Moisture Co	ondition			VN Vein		In eguidi
Γ	D	Dry				
	Μ	Moist		Infill or Coating	Roughn	
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	Wp	Plastic Limit		Cl Clay	slk	Slickensided
		Liquid Limit		Ca Calcite	smo	Smooth
1	MC	Moisture Content		Fe Iron oxi	de rou	Rough
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Density In		277 B	I _D (%)	R	Roller/Tricone	
	VL	Very Loose	< 15	W	Washbore	
	L	Loose	15 - 35		denotes bit shown	by suffix
	MD	Medium Dense	35 - 65	В	Blank Bit	
	D	Dense	65 - 85	V	"V" Shaped Bit	
	VD	Very Dense	> 85	Т	Tungsten Carbi	
				LB	Large Bore Pus	
				MC	Macro Core Pus	
				DT	Dual Push Tube	e Drilling
ROCK DE	SCRIPTIONS			NMLC	NMLC Core Dril	ling
	20 - 1962-2062-00 - 59737-5975-5-50			NQ/HQ	Wireline Core D	
Weatherin						
	Rs	Residual Soil		С	Casing	
	XW	Extremely Weathere	d	М	Mud	
	HW	Highly Weathered				
	MW	Moderately Weather		SAMPLES/TEST	S	
	DW	Distinctly Weathered	1			
	SW	Slightly Weathered		В	Bulk sample	513-05
	FR	Fresh		D	Disturbed samp	
	(DW covers b	oth HW & MW)		U50	Thin-walled tub	A CONTRACTOR OF
<u>.</u>			x /Fo> ···-		(50mm diamet	
Strength	220	20 T T T	Is (50) MPa	PP	Pocket penetro	
	EL	Extremely Low	< 0.03	N*	SPT (blows per	
	VL	Very Low	0.03 - 0.1		*denotes sam	 Post Dynamics and and an exceptions.
				Nc	SPT with solid	
	L	Low	0.1 - 0.3			cone
	Μ	Medium	0.3 - 1	R	SPT refusal	cone
	M H	Medium High	0.3 - 1 1 - 3	R	SPT refusal	cone
	M H VH	Medium High Very High	0.3 - 1 1 - 3 3 - 10		SPT refusal	cone
	M H	Medium High	0.3 - 1 1 - 3	R	SPT refusal	cone
Structure	M H VH EH	Medium High Very High	0.3 - 1 1 - 3 3 - 10 > 10	R VANE SHEAR TE	SPT refusal	
Structure	M H VH EH	Medium High Very High Extremely High	0.3 - 1 1 - 3 3 - 10	R	SPT refusal ESTS Vane shear str	rength
Structure	M H VH EH	Medium High Very High Extremely High	0.3 - 1 1 - 3 3 - 10 > 10 Spacing	R VANE SHEAR TE	SPT refusal	ength (kPa) and
Structure	M H VH EH Thinly Lamina Laminated	Medium High Very High Extremely High ated	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm	R VANE SHEAR TE	SPT refusal ESTS Vane shear str Peak/residual	ength (kPa) and
Structure	M H VH EH Thinly Lamin: Laminated Very thinly b	Medium High Very High Extremely High ated edded	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm 6 - 20mm	R VANE SHEAR TE	SPT refusal ESTS Vane shear str Peak/residual Vane size (mm	ength (kPa) and
Structure	M H VH EH Thinly Lamina Laminated	Medium High Very High Extremely High ated edded d	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm 6 - 20mm 20 - 60mm	R VANE SHEAR TE Su WATER MEASU	SPT refusal ESTS Vane shear str Peak/residual Vane size (mm	rength (kPa) and))
Structure	M H VH EH Thinly Lamina Laminated Very thinly b Thinly bedde	Medium High Very High Extremely High ated edded d	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm 6 - 20mm 20 - 60mm 60 - 200mm	R VANE SHEAR TE Su	SPT refusal ESTS Vane shear str Peak/residual Vane size (mm REMENTS	rength (kPa) and))
Structure	M H VH EH Thinly Lamina Laminated Very thinly b Thinly bedde	Medium High Very High Extremely High ated edded d ded	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm 6 - 20mm 20 - 60mm 60 - 200mm	R VANE SHEAR TE Su WATER MEASU	SPT refusal ESTS Vane shear str Peak/residual Vane size (mm REMENTS Water level at	rength (kPa) and))
Structure	M H VH EH Thinly Lamina Laminated Very thinly b Thinly bedde Medium bedd	Medium High Very High Extremely High ated edded d ded	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm 6 - 20mm 20 - 60mm 60 - 200mm 0.2 - 0.6m	R VANE SHEAR TE Su WATER MEASU	SPT refusal ESTS Vane shear str Peak/residual Vane size (mm REMENTS Water level at	rength (kPa) and h) the time of
Structure NOTE:	M H VH EH Thinly Lamin Laminated Very thinly b Thinly bedde Medium bedd Thickly bedd Very thickly	Medium High Very High Extremely High ated edded d ded ed bedded	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm 6 - 20mm 20 - 60mm 60 - 200mm 0.2 - 0.6m 0.6 - 2.0m > 2.0m	R VANE SHEAR TE Su WATER MEASU	SPT refusal ESTS Vane shear str Peak/residual Vane size (mm REMENTS Water level at drilling	rength (kPa) and h) the time of
	M H VH EH Thinly Lamin Laminated Very thinly b Thinly bedde Medium bedd Thickly bedd Very thickly	Medium High Very High Extremely High ated edded d ded	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm 6 - 20mm 20 - 60mm 60 - 200mm 0.2 - 0.6m 0.6 - 2.0m > 2.0m	R VANE SHEAR TE Su WATER MEASU	SPT refusal ESTS Vane shear str Peak/residual Vane size (mm REMENTS Water level at drilling Water level at Water inflow	rength (kPa) and 1) the time of fter drilling
	M H VH EH Thinly Lamin: Laminated Very thinly be Medium bedde Medium bedde Thickly bedd Very thickly bedd Very thickly bedd	Medium High Very High Extremely High ated edded d ded ed bedded	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm 6 - 20mm 20 - 60mm 60 - 200mm 0.2 - 0.6m 0.6 - 2.0m > 2.0m	R VANE SHEAR TE Su WATER MEASU	SPT refusal ESTS Vane shear str Peak/residual Vane size (mm REMENTS Water level at drilling Water level at	rength (kPa) and 1) the time of fter drilling
NOTE:	M H VH EH Thinly Lamin: Laminated Very thinly be Medium bedde Medium bedde Thickly bedd Very thickly bedd Very thickly bedd	Medium High Very High Extremely High ated edded d ded ed bedded	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm 6 - 20mm 20 - 60mm 60 - 200mm 0.2 - 0.6m 0.6 - 2.0m > 2.0m	R VANE SHEAR TE Su WATER MEASU	SPT refusal ESTS Vane shear str Peak/residual Vane size (mm REMENTS Water level at drilling Water level al Water inflow Water outflow	rength (kPa) and 1) the time of fter drilling
NOTE:	M H VH EH Thinly Lamini Laminated Very thinly bedde Medium bedd Thickly bedd Very thickly bedd Very thickly bedd Soil And roo - 1993	Medium High Very High Extremely High ated edded d ded ed bedded	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm 6 - 20mm 20 - 60mm 60 - 200mm 0.2 - 0.6m 0.6 - 2.0m > 2.0m	R VANE SHEAR TH SU WATER MEASU T T A - A	SPT refusal ESTS Vane shear str Peak/residual Vane size (mm REMENTS Water level at drilling Water level al Water inflow Water outflow SILLING	rength (kPa) and 1) the time of fter drilling Factor
NOTE:	M H VH EH Thinly Lamini Laminated Very thinly bedde Medium bedd Thickly bedd Very thickly bedd Very thickly bedd Soil And roo - 1993	Medium High Very High Extremely High ated edded d ded ed bedded	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm 6 - 20mm 20 - 60mm 0.2 - 0.6m 0.6 - 2.0m > 2.0m sed on AS 1726	R VANE SHEAR TH Su WATER MEASU V C C C C C C C C C C C C C C C C C C	SPT refusal ESTS Vane shear str Peak/residual Vane size (mm REMENTS Water level at drilling Water level at Water inflow Water outflow SILLING ance	rength (kPa) and n) the time of fter drilling Factor 0
NOTE: Natural F	M H VH EH Thinly Lamin: Laminated Very thinly be Medium bedde Medium bedde Thickly bedd Very thickly bedd Very thickly bedd Very thickly bedd Soil And row - 1993	Medium High Very High Extremely High ated edded d bedded ck descriptions are ba Shape pl	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm 6 - 20mm 20 - 60mm 60 - 200mm 0.2 - 0.6m 0.6 - 2.0m > 2.0m sed on AS 1726	R VANE SHEAR TH Su WATER MEASU V V V V V V V V V V V V V V V V V V V	SPT refusal ESTS Vane shear str Peak/residual Vane size (mm REMENTS Water level at drilling Water level at Water inflow Water outflow Mater outflow ILLLING ance percussion	rength (kPa) and 1) the time of fter drilling Factor 0 1
NOTE: Natural F Type	M H VH EH Thinly Lamin Laminated Very thinly be Medium bedde Medium bedde Thickly bedd Very thickly l Soil And roo - 1993	Medium High Very High Extremely High ated edded d bedded ck descriptions are ba Shape pl	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm 6 - 20mm 20 - 60mm 0.2 - 0.6m 0.6 - 2.0m > 2.0m sed on AS 1726 Planar Curved	R VANE SHEAR TH Su WATER MEASU WATER MEASU WATER MEASU WATER MEASU WATER MEASU MALL MEASU MALL MEASU No percussion Fast push with p Medium push with p	SPT refusal ESTS Vane shear str Peak/residual i Vane size (mm REMENTS Water level at drilling Water level at Water inflow Water outflow Water outflow SILLING sance vercussion th percussion	rength (kPa) and 1) the time of fter drilling Factor 0 1 2
NOTE: Natural F Type JT	M H VH EH Thinly Lamin: Laminated Very thinly be Medium bedde Medium bedde Thickly bedd Very thickly bedd Very thickly bedd Very thickly bedd Soil And row - 1993	Medium High Very High Extremely High ated edded d bedded ck descriptions are ba Shape pl	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm 6 - 20mm 20 - 60mm 60 - 200mm 0.2 - 0.6m 0.6 - 2.0m > 2.0m sed on AS 1726	R VANE SHEAR TH Su WATER MEASU W J Su Noter MEASU No PUSH TUBE DR Degree of Resist No percussion Fast push with p Medium push with	SPT refusal SPT refusal Vane shear str Peak/residual Vane size (mm REMENTS Water level at drilling Water level at Water inflow Water outflow Water outflow SILLING ance Percussion th percussion percussion	rength (kPa) and 1) the time of fter drilling Factor 0 1 2 3
NOTE: Natural F Type JT BP	M H VH EH Thinly Lamin: Laminated Very thinly bedde Medium bedde Thickly bedd Very thickly bedd Very thickly bedd Very thickly bedd Fractures	Medium High Very High Extremely High ated edded ded ed bedded ck descriptions are ba Shape pl ne cu	0.3 - 1 1 - 3 3 - 10 > 10 Spacing < 6mm 6 - 20mm 20 - 60mm 0.2 - 0.6m 0.6 - 2.0m > 2.0m sed on AS 1726 Planar Curved	R VANE SHEAR TH Su WATER MEASU WATER MEASU WATER MEASU WATER MEASU WATER MEASU MALL MEASU MALL MEASU No percussion Fast push with p Medium push with p	SPT refusal SPT refusal Vane shear str Peak/residual Vane size (mm REMENTS Water level at drilling Water level at Water inflow Water outflow Water outflow SILLING ance Percussion th percussion percussion	rength (kPa) and 1) the time of fter drilling Factor 0 1 2

December 2007



TEST REPORT

Client:	DY Surveyors	Job No:	W07/1966	Sheet:	1 of 1
Principal:	-				
Project:	Proposed Subdivision	Tested By:	PB	Date:	28.11.12
Location:	Downs Place, Jamberoo	Checked By:	HU	Date:	4.12.12

Sample Procedure:

AS1289.1.2.1 (Cl 6.5.4) - Machine Excavated Pit/Trench

MOISTURE CONTENT - AS1289 2.1.1

Sample Number	Sample Description	Sample Location	Test Results
W39873	CLAY	TP1 (0.3 - 0.6m)	19.4%
W39876	CLAY	TP4 (0.5 - 0.7m)	16.2%
W39874	Sandstone	TP2 (1.2 - 1.3m)	21.1%
W39875	CLAY	TP2 (0.4 - 0.6m)	12.3%

REMARKS:



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APPROVED SIGNATORY Harry Ubungen DATE 4/12/12

Document No. RP132-1 Version 3 22-6-10

Wollongong Laboratory 1318



TEST REPORT

ACN 069 211 561 Unit 1/140 Industrial Road Oak Flats,NSW,2529,AUSTRALIA Telephone 61 2 4257 4458 Fax 61 2 4257 4463 Email: southcoast@netgeo.com.au

		5
Client:	DY Surveyors	Job No:
Project:	Proposed Residential Subdivision	Report N
Location:	Jamberoo	Report [
GTR Number:		Tested E

Sheet:	1 of 2
Job No:	W07/1966
Report No:	1
Report Date:	4/12/2012
Tested By:	Philip Baltoski

Sample Identification				
Sample Description : CL ⁴	Y, brown	Sampling Procedure:	AS1289.1.2.1(C Excavated Pit T	· · · · · · · · · · · · · · · · · · ·
Sample Number:	TP1 (0.3 - 0.6m)	Date Sampled:	23/11/2012	
Laboratory Number:	W39873	Sample History:	Oven Dried	
Client Number:	-	Preparation Method:	Dry	
		Shrinkage Mould Length:	250	(mm)

ATTERBERG LIMITS & LINEAR SHRINKAGE

TEST PROCEDURE		TEST RESULTS	SPECIFICATION
Liquid Limit (W _L) AS1289.3.1.1	%	68	
Plastic Limit (W _P) AS1289.3.2.1	%	26	
Plasticity Index (I _P) AS1289.3.3.1	%	42	
Linear Shrinkage(L _S) AS1289.3.4.1	%	17.0	
Moisture Content Method: AS1289.2.1.1			

REMARKS:



Accredited for compliance with ISO/IEC 17025.

Wollongong Laboratory 1318

APPROVED SIGNATORY Harry Ubungen



TEST REPORT

ACN 069 211 561 Unit 1/140 Industrial Road Oak Flats,NSW,2529,AUSTRALIA Telephone 61 2 4257 4458 Fax 61 2 4257 4463 Email: southcoast@netgeo.com.au

Client:	DY Surveyors
Project:	Proposed Residential Subdivision
Location:	Jamberoo
GTR Number:	

Sheet:	2 of 2
Job No:	W07/1966
Report No:	1
Report Date:	4/12/2012
Tested By:	Philip Baltoski

Sample Identification AS1289.1.2.1(Cl6.5.4) Machine Sampling Procedure: Sample Description : Excavated Pit Trench CLAY, brown TP4 (0.5 - 0.7m) 23/11/2012 Sample Number: Date Sampled: W39874 Oven Dried Laboratory Number: Sample History: Client Number: Preparation Method: Dry _ Shrinkage Mould Length: 250 (mm)

ATTERBERG LIMITS & LINEAR SHRINKAGE

TEST PROCEDURE		TEST RESULTS	SPECIFICATION
Liquid Limit (W _L) AS1289.3.1.1	%	85	
Plastic Limit (W _P) AS1289.3.2.1	%	25	
Plasticity Index (I _P) AS1289.3.3.1	%	60	
Linear Shrinkage(L _S) AS1289.3.4.1	%	20.5	
Moisture Content Method: AS1289.2.1.1			

REMARKS:



Accredited for compliance with ISO/IEC 17025.

Wollongong Laboratory 1318

APPROVED SIGNATORY Harry Ubungen DATE 4/12/2012

$\begin{array}{c} & & & & & & & & \\ & & & & & & & \\ & & & & & \\ & &$	$\frac{1}{2} \frac{1}{2} \frac{1}$	ACE
	Scale: A4 - NOT TO SCALE	Client: D.Y. Surveyors
12/9-15 Gundah Rd, Mt. Kuring-Gai NSW 2529 Tel: (02) 8438 0312 Fax: (02) 8438 0310 Fax: (02) 8438 0310	Date: 6/12/12	Project: Proposed Residential Subdivision
Geotechnics Email: masadabadi@netgeo.com.au	Drawing: MA	Location: LOT 100 & 101 DP 1157883, Downes Place, JAMBEROO
	Drawing No: W07/1966-1	SITE PLAN